

## BRYANT Chassis 1 & 2:

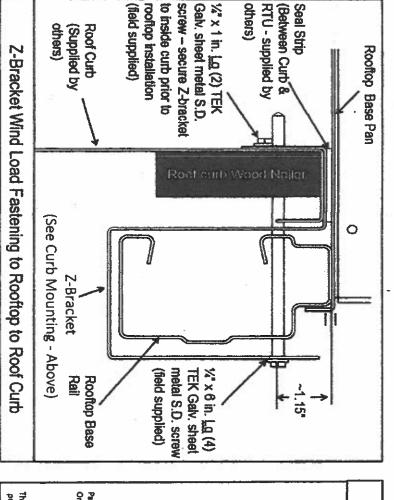
Models: 582J/559J, 559K, 547K, and 548J - size 04 (min) through 07(max) 580J/558J, 581J/551J, 549J and 549K - size 04(min) through 06 (max)

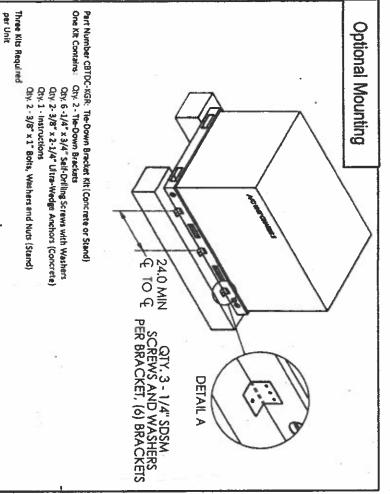
Each package unit air conditioner listed above conforms to the Florida Building Code  $7^{th}$  Edition (2020) requirements for installation including High Velocity Hurricane Zone (HVHZ), Risk Category III (V = 186 MPH), exposure category "D", and installation height up to and including 65 feet above grade. Worst case is -07 (chassis 2) 74-3/8"  $\times$  46-3/4"  $\times$  41-3/8" tall.

## ALLOWABLE DESIGN PRESSURES FOR THE UNIT ITSELF:

Design Lateral Pressure = 197.2 lb/ft² Design Uplift Pressure = 95.4 lb/ft²

other suitable mounting arrangement and all factory supplied assembly fasteners are in place. provided the 16 gage galvanized base rails are properly fastened to a suitable slab, curb, curb adapter, or Unit itself will withstand wind loads imposed by 197.2 PSF lateral and 95.4 PSF uplift design pressures







Job No. Chassis 1 & 2

Date: 01/12/21

Created by: J. Buerosse

Title

Job No.:

Model List and Details

**Bryant Rooftop Units** 

```
Since positive pressure acts toward the surface being considered and negative pressure acts away, only the uplift pressure will remove a panel from the machine. The design lateral pressure which is considered to act toward the windward surface is recognized to be a combination of the pressures acting on the windward and leeward surfaces. Wall pressure coefficients from ASCE7-16, Chapter 27,
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                Design Pressures complying to FBC Building 1620.6 (HVHZ): V = 186 \text{ mph} (Risk Cat. III), For Exp. Cat. "D" and Z = 65 \text{ ft}, K_s = 1.33, K_m = 1.0, K_d = 0.90 Q_s = .00256K_sK_sK_sV^2 = 106.01 \text{ lb/ft}^3
                                                                                                                                                                                                                                                                                                   Design Uplift Pressure = U_L(0.6) = 95.4 \text{ lb/ft}^3
                                                                                                                                                                                                                                                                                                                                                                                  Design Lateral Pressure = W_L(0.6) = 197.2 \text{ lb/ft}^3
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                  Factoring in the required Load Combination factor (0.6):
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 Lateral Wind Pressure = W_L = q_c(3.1) = 328.64 \text{ lb/ft}^3
Uplift Wind Pressure = U_L = q_c(1.5) = 159.02 \text{ lb/ft}^3
                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                         Rational Analysis: Worst case is -07 (Chassis 2) 74-3/8" x 46-3/4" x 41-3/8"
```

Worst case positive pressure coefficient is 0.8 for windward wall which has a corresponding negative pressure coefficient of 0.5 on the leeward wall. The worst case negative pressure coefficient is 0.7 for the sidewall (side parallel to wind). Since the windward and leew wall pressures act in the same direction, the distibuted pressures are computed as follows: L/B = 46.75/74.375 = 0.63 for wind on long (74-3/8") side L/B = 74.375/46.75 = 1.59 for wind on short (46-3/4") side Figure 27.3-1 may be used to distribute the Design Lateral Pressure into positive and negative componenets acting on the windward and Since the windward and leeward

Lateral Positive Design Pressure =  $197.18 (0.8)/(0.8 + 0.5) = 121.34 \text{ lb/ft}^2$  (Worst Case Positve) Lateral Negative Design Pressure =  $197.18 (0.5)/(0.8 + 0.5) = 75.84 \text{ lb/ft}^2$  (Worst Case Negative) Sidewall Negative Design Pressure =  $197.18 (0.7)/(0.8 + 0.5) = 106.17 \text{ lb/ft}^2$  (Worst Case Negative)

based upon ICC-ES Report ESR-2196; 22 ga. panels and columns are fastened together and to 16 ga. base rails using #10 serrated washer head self piercing screws having 425" head diameter, 0.19" nominal diameter, and 0.14 minor diameter. These screws are expected to exhibit the following properties

```
Pullover strength of 22 ga. = 828 lbs (ultimate)
Shear Strength in 22 ga. = 684 lbs (ultimate)
                                                                                     Pullout Strength in 22 ga. = 306 lbs (ultimate)
```

Pullout Strength in 16 ga. = 450 lbs (ultimate - based upon 18 ga.) Shear Strength in 16 ga. = 927 lbs (ultimate - based upon 18 ga.)

73.6" x 45" draw formed panel anchored at edges and through top to center panel and control box. Worst case portion is over air handler section since condenser section has a large hole in the top causing internal and external pressure to be equal. For portion tributary to air handling section:

```
For outside edge (7 screws, all in shear), screw load = 1150.9/2(7) = 82.2 lbs
Safety Factor = 684/82.2 = 8.3 OK
                                                For inside edge (8 screws, 4 in tension), screw load = 1150.9/2(8) = 71.9 lbs
Safety Factor = 306/71.9 = 4.3 OK
                                                                                                                                                                                                                          Load = 12.06 (95.41) = 1150.9 lbs
                                                                                                                                                                                                                                                                        A = 45(38.6)(12(12) = 12.06 \text{ ft}^2
For Inside Panel (50HJ540465):
                                             Safety Factor = 306/71.9 = 4.3
```

through flange perpendicular to face (10 screws in tension, 10 screws in shear). A = 44.84(37.53 y 12(12) = 11.69 ft<sup>2</sup> Load = 11.69(106.17) = 1240.7 lbs Screw Load = 1240.7/20 = 62.04 lbs 44.84" x 37.53" draw formed penel anchored at edges with 5 screws through face at top and bottom and 5 screws each vertical edge

Safety: Factor = 306/62.04 = 4.9

33.5"  $\times$  36.5" draw formed panel anchored with 2 screws through face each vertical side, 3 screws through face at bottom edge and top For Access Door ( 48TM500284):

edge fits inside top penel (trapped).

Screw Load = 901.5/2(5) = 90.15 lbs Load = 8.49(106.17) = 901.5 lbs

OK for Components and Cladding

Screw Load = 333.6/2(3) = 55.60 lbs edge fits inside top panel (trapped).  $A = 12.13(37.3)/12(12) = 3.14 \text{ ft}^2$ For Access Panel (48TM500345): Load = 3.14(106.17) = 333.6 lbs12.13" x 37.3" draw formed panel anchored with 1 screw through face each vertical Safety Factor = 306/55.60 = 5.5 OK for Components and side, I screw through face at bottom edge and top Cledding

Remaining panels are trivial cases of the above due to greater fastener quantity or having openings that limit negative pressure effects.

For connection of upper frame and panels to base rails:

columns to 16 ga. base rails. handler end. Opposite end is louvered and has a large opening in the top and mesh 12 screws each long side fasten frame columns and panels to the long base rails. 5: over cooling coils. crews fasten inside panel to short base rail at air

Uplift Wind Area =  $A_0 = 73.6(45)/12(12) = 23.0 ft^3$ Uplift Design Load = 23.0(95.41) = 2194 lbs Uplift Moment = 2194(45)/2 = 49375 in-lb Overturning Moment = 3782(37.53)/2 = 70975 in-lb Lateral Wind Area = A<sub>L</sub> = 73.6(37.53)/12(12) = 19.18 ft<sup>3</sup> Lateral Design Load = 19.18(197.18) = 3782 lbs

Screw Load = (70975 + 49375)/12(45) = 222.9 lbs (shear) Safety Factor = 927/222.9 = 4.2

supplied assembly fasteners are in place. base rails are properly fastened to a suitable slab, stand, curb, curb adapter, or other s Unit itself will withstand wind loads imposed by 197.2 psf lateral and 95.4 psf uplift design pressures provided the 16 gage galvanized attable mounting arrangement and all factory

For connection of unit base rails to properly designed curb, metal stand, or structural concrete (by others):

Lateral Design Load = 21.37(197.18) = 4214 lbs
Overturning Moment = 4214(41.375)/2 = 87172 in-lb
Uplift Wind Area = A<sub>11</sub> = 74.375(46.75)/12(12) = 24.15 ft<sup>2</sup>
Uplift Design Load = 24.15(95.41) = 0.6(607) = 1940 lbs Uplift Moment = 1940(46.75)/2 = 45348 in-lbLateral Wind Area =  $A_L = 74.375(41.375)12(12) = 21.37 ft^2$ 

drilling screws: For connection of 16 ga. (min) straps, clips, or brackets spaced 48" min apart to Report ESR - 1976 unit base rails on long sides using 1/4" (#14) self-

These screws are expected to exhibit the following properties based upon ICC - ES Pullout Strength in 16 ga. = 573 lbs (ultimate)

Shear Strength in 16 ga. = 1389 lbs (ultimate)

Using 3 screws per strap, clip, or bracket, with 3 straps, clips, or brackets each long side:

Screw Load = (87172 + 45348)/2(3)(46.75) = 315.0 lbs (shear) at base rail outer surface

Safety Factor = 1389/315.0 = 4.4

OK for Components and Cladding

18 ga. (min) curb (by others): For Z-Brackets similar to Micrometi design but modified to eliminate hidden structural fasteners (see sheets 2 and 3) anchored to

Shear Strength in 18 ga. = 1218 lbs (ultimate)
Screw Load = (87172 + 45348)/2(4)(42.69) = 388.0 lbs (shear) at curb inside surface Safety Factor = 1218/388.0 = 3.1OK for Components and C Cladding

brackets each long side: Anchor Load = (87172 + 45348)/3(47.5) = 930.0 lbs (tension) For brackets 3.25" wide x 2" x 2-1/2", 16 ga. (min) spaced 24" (min) on center into base rails, Using(3) screws per bracket, (3)

For 3/8" SAE Gr. 5 bolts with nuts and washers to steel (by others) Safety Factor = 1937/702.3 = 2.8 (shear) Safety Factor = 3720/930.0 = 4.0 (tension)

Anchor Load = 4214/6 = 702.3 lbs (shear) at 3/4" beyond baserail outer

3/4" (min) edge distance, and 2-1/2" (min) spacing: For 3/8" Powers Wedge-Bolt + anchors with 2-1/8" (min) embedment into 2000 psi (min) concrete (by others), 4" (min) thick, 2-

Safety Factor = 3000/930.0 = 3.2 (tension) OK Safety Factor = 3100/702.3 = 4.4 (shear) OK

Chassis 1 &2 Job No. Job No.: **Bryant Rooftop Units S2** 01/12/21 Date: Title Model List and Details Created by: J. Buerosse

John Buerosse, Structural and Mechanical Engineering 750/E. Sample Road - Bldg. 3 - #220 Pompano Beach, FL 33064 Tel: 954-633-4692 Fax: 954-784-004